An efficient analytical solution for nonlinear vibrations of a parametrically excited beam

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Abstract

An efficient and accurate analytical solution is provided using the homotopy-Pade technique for the nonlinear vibration of parametrically excited cantilever beams. The model is based on the Euler-Bernoulli assumption and includes third order nonlinear terms arisen from the inertial and curvature nonlinearities. The Galerkin's method is used to convert the equation of motion to a nonlinear ordinary differential equation, which is then solved by the homotopy analysis method (HAM). An explicit expression is obtained for the nonlinear frequency amplitude relation. It is found that the proper value of the so-called auxiliary parameter for the HAM solution is dependent on the vibration amplitude, making it difficult to rapidly obtain accurate frequency-amplitude curves using a single value of the auxiliary parameter. The homotopy-Pade technique remedied this issue by leading to the approximation that is almost independent of the auxiliary parameter and is also more accurate than the conventional HAM. Highly accurate results are found with only third order approximation for a wide range of vibration amplitudes.

1. Introduction

The vibration of a beam subjected to the harmonic base excitation is of high importance because of the wide application of such a structure in many fields of engineering as manipulator arms, offshore flexible structures and space structures0 [1]. Many theoretical and experimental studies have been performed in this area since 1971 [2-8]. Because of the complexity of the governing nonlinear equations, and the need for rapid estimation of the amplitude dependent frequencies, numerous attempts have been made to obtain an accurate analytical solution of the problem [1, 6, 7]. Different methods such as the method of multiple scales and harmonic balance are used in these studies and are shown to be effective and accurate. These methods, however, loose their accuracy as the nonlinearity increases unless their higher order approximations are used. This may not be accomplished in a convenient and systematic manner, and in most cases requires heavy mathematical manipulations or numerical treatments to solve many nonlinear algebraic equations.

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New analytical methods have been introduced in recent years, which do not depend on the parameters such as the Adomian decomposition, Homotopy Perturbation, He's parameter expanding (HPEM), Variational iteration, Max–Min approach (MMA), Iteration perturbation (IPM), and the Homotopy analysis (HAM) methods [9]. An active research area has been opened in recent years to demonstrate the reliability and accuracy of these analytical methods for different engineering applications. The HPEM was used by Sedighi and Shirazi [10]0 for studying the vibration of a cantilever beam with nonlinear boundary conditions, and also by Sedighi et al. [11] for vibrations of a beam with preload discontinuity. Application of some of the above mentioned analytical methods in the nonlinear vibration of beams was also considered in Refs. [9, 12-14]. In all of these studies, it has been demonstrated that the new modern techniques may be very helpful in providing analytical solutions for the vibration of structural systems possessing strong nonlinearities. Among these methods, the HAM has also been proved to be easy and accurate for treating nonlinear vibration problems [15-22]. One of the main advantages of this method over many other analytical methods mentioned above is that the convergence of the series solution obtained by the method can be guaranteed using the so-called auxiliary parameter. It is in fact shown in Ref 0 that the convergence rate can be considerably improved by choosing a proper value for the auxiliary parameter. The proper value of this auxiliary parameter can be determined by visually inspecting the so-called h-curves. However, this may slow down the solution process in cases that the frequency-response (backbone) curves are intended to be plotted. The reason is that the proper value of the auxiliary parameter may not be the same for different vibration amplitudes. Hence although the auxiliary parameter would be beneficial in terms of controlling the convergence rate, it may also slow down the method if the backbone curves are needed to be plotted. This drawback is shown in the present study that can be removed if the Pade approximant is employed. The combination of the Pade approximant and the HAM is used by Liao and Cheung 0 under the name of the homotopy-Pade technique and is shown to have a better convergence rate than the HAM.

Due to the capabilities of the HAM and the homotopy-Pade technique mentioned above, they are used in the present study to provide a convergent analytical solution for the nonlinear vibration of a parametrically excited beam. The equation of the motion is based on the Euler-Bernoulli's assumption with the order of three nonlinearity. The solution process is initiated by discretizing the integro-partial differential equation of motion using the Galerkin's method. The resulted nonlinear ordinary differential equation is then solved by the HAM and the homotopy-Pade technique. It is found that the homotopy-Pade technique has a superior performance over the HAM since the corresponding solution has faster convergence and minimal dependence on the auxiliary parameter. The results are compared using numerical solution to show high accuracy and efficiency of the method for a wide range of vibration amplitudes. It is worth to mention that the solution method used in the present study can, in fact, be used for strongly nonlinear vibration analysis of any structural systems like plates or beam assemblies, as long as their motion can be described by a single mode. However, in cases that more than one mode is required to accurately predict the nonlinear vibration of the system, the method may not always yield accurate results especially when the nonlinear interactions occur between the modes due to internal resonances.

2. Governing equation

Consider a cantilever beam shown in Fig. 1, with the length, l, the mass per unit length, ρ and the flexural rigidity, EI, which is attached at its base, to a rigid mass having a harmonic motion of frequency Ω and amplitude b. The integro-partial differential equation of motion that describes the moderately large amplitude

vibration of the beam can be written in the following non-dimensional form [1, 23]:

$$\frac{\partial^2 w}{\partial \tau^2} + \frac{\partial^4 w}{\partial s^4} + \frac{\partial}{\partial s}[\frac{\partial w}{\partial s} \cdot \frac{\partial}{\partial s}(\frac{\partial w}{\partial s}\frac{\partial^2 w}{\partial s^2})] +$$

$$\frac{1}{2}\frac{\partial w}{\partial s}\int_0^s \frac{\partial^2 \left[\int_0^{s_1} (\frac{\partial w}{\partial s})^2 ds_2 \right]}{\partial \tau^2} ds_1 = \quad (1)$$

$$[b_0 \Omega_0^2 \cos(\Omega \tau) - g_0][(1-s)\frac{\partial^2 w}{\partial s^2} - \frac{\partial w}{\partial s}],$$

where $\tau = (EI/\rho L^4)^{1/2}t$, $\Omega_0 = l^2(\rho/EI)^{1/2}\Omega$, $w = \bar{w}/l$ with $\bar{w}$ being the transverse deflection, $b_0 = b/l$, and $s = x/l$. Also, denoting the gravitational constant by g, g_0 is defined by $g_0 = g(\rho l^3/EI)$. It is to be noted that the nonlinear equation of motion of the beam given in Eq. (1) are derived in Ref. 0 based on the generally large deformation of the beam, which is then simplified to contain only up to the third-order nonlinear terms. Moreover, since the beam is not constrained in the axial direction, the beam is assumed to be inextensible. The warping, shear deformation and also the rotary inertial of the cross section of the beam are also neglected due to the small thickness of the beam.
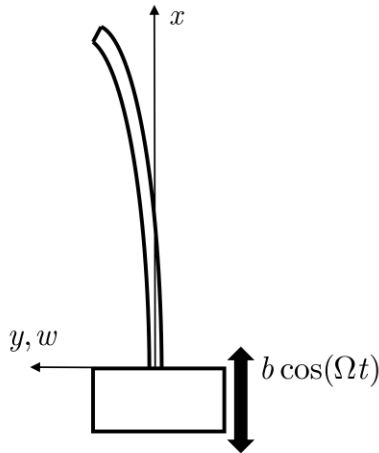

Fig. 1. Cantilever beam subjected to base excitation.

In order to solve Eq. (1), the Galerkin's method is used at first to convert it to an ordinary differential equation. The displacement function used for this purpose corresponds to the exact modes of the linear vibration of a cantilever beam, which is defined as:

$$\phi_i(s) = \cosh(\beta_i s) - \cos(\beta_i s) +$$

$$\frac{\cosh(\beta_i) + \cos(\beta_i)}{\sinh(\beta_i) + \sin(\beta_i)}[\sinh(\beta_i s) - \sin(\beta_i s)], \quad (2)$$

with β_i being the ith root of the transcendental equation, $\cos(\beta)\cosh(\beta) + 1 = 0$. Assuming that the motion of the beam is dominated by a single mode, the single-mode Galerkin's procedure may be used for discretization purpose [1]. This assumption may not be acceptable if the equations of motion contain quadratic nonlinear terms [24] or internal resonance occurs between the modes of the beam. Considering that no quadratic terms are present in Eq. (1) and also since it is assumed that the frequencies are not commensurate (or nearly commensurate) with each other, the single linear mode may accurately describe the motion.

Next, introducing the relation, $w = \phi_i(s)v(\tau)$ into Eq. (1), applying the Galerkin's procedure and defining the dimensionless parameters, $t^* = \theta^2\tau$, $\omega = \Omega_0/2\theta^2$ and $u = \theta v$, the following equation is obtained as [1]:

$$\frac{\partial^2 u}{\partial t^{*2}} + [1 + 2p\cos(2\omega t^*)]u +$$

$$\alpha_1 u \frac{\partial^2}{\partial t^{*2}}(u^2) + \alpha_2 u^3 = 0, \quad (3)$$

where the constant coefficients θ, α_1, α_2 and p are dependent on the system properties.

3. Solution by HAM

The HAM begins with introducing the transformation, $T = \omega t^*$, into Eq. (3) as follows:

$$\omega^2 \frac{\partial^2 u}{\partial T^2} + [1 + 2p\cos(2T)]u + \quad (4)$$

$$\alpha_1 \omega^2 u \frac{\partial^2}{\partial T^2}(u^2) + \alpha_2 u^3 = 0.$$

The so-called zeroth order deformation equation is then constructed as 0:

$$(1-q)L[\phi(T;q)-u_0(T)] = qhN[\phi(T;q)], \qquad (5)$$

where u_0 is the zeroth order solution, q is the embedding parameter that varies from 0 to 1, and h is the auxiliary parameters to be determined later. Also, N is a nonlinear operator that is defined for the present problem as follows:

$$N[\phi(T;q),\Omega(q)] = \Omega(q)^2 \frac{\partial^2\phi(T;q)}{\partial T^2} +$$
$$[1+2p\cos(2T)]\phi(T;q) + \qquad (6)$$
$$\alpha_1\Omega(q)^2\phi(T;q)\frac{\partial^2}{\partial T^2}\left[\phi(T;q)^2\right] + \alpha_2\phi(T;q)^3,$$

where $\Omega(q)$ and $\phi(T;q)$ are unknown mapping functions that satisfy the following relations:

$$\phi(T;0) = u_0(T), \qquad \Omega(0) = \omega_o,$$
$$\phi(T;1) = u(T), \quad \Omega(1) = \omega, \qquad (7)$$

with ω_o being the first order solution for the non-dimensional frequency. L in Eq. (5) is also the linear operator which is defined as:

$$L(\phi) = \frac{\partial^2\phi}{\partial T^2} + \phi, \qquad (8)$$

The above linear operator is chosen such that its homogeneous solution would be in the form of the functions that appear in the base function of the solution. For vibration problems with periodic solution, the base function can be represented by the series, $\sum_{n=1}^{\infty} c_n \cos(nt)$ whose first term is $\cos(t)$. Hence it would be reasonable to define the linear operator by Eq. (8), since its homogeneous solution is also $\cos(t)$.

Taylor's expansion of the unknown functions, $\Omega(q)$ and $\phi(T;q)$ in terms of the embedding parameter, q, are then obtained as:

$$\phi(T;q) = \phi(T;0) + \sum_{m=1}^{\infty} \left.\frac{\partial^m\phi(T;q)}{m!\partial q^m}\right|_{q=0} q^m, \qquad (9)$$

$$\Omega(q) = \Omega(0) + \sum_{m=1}^{\infty} \left.\frac{\partial^m\Omega(q)}{m!\partial q^m}\right|_{q=0} q^m. \qquad (10)$$

Next, using the definition, $u_m(T) = \left.\dfrac{\partial^m\phi(T;q)}{m!\partial q^m}\right|_{q=0}$ and

$\omega_m = \left.\dfrac{\partial^m\Omega(q)}{m!\partial q^m}\right|_{q=0}$ along with Eq. (7), the following expression can be obtained for ω and $u(T)$ as:

$$\omega = \omega_0 + \sum_{m=1}^{\infty} \omega_m,$$
$$\qquad (11)$$
$$u(T) = u_0(T) + \sum_{m=1}^{\infty} u_m(T).$$

Considering the initial condition, $u(0) = A$ and $\left.\dfrac{du}{dT}\right|_{T=0} = 0$, the zeroth order solution $u_0(T)$ is taken as:

$$u_0(T) = A\cos(T), \qquad (12)$$

which is the solution of the homogeneous linear equation given in Eq. (8). The remaining unknowns, ω_i's and u_i's will be determined using the higher-Order deformation equations. These equations can be obtained by differentiating the zeroth-order deformation equation m times with respect to q, dividing the result by $m!$ and finally setting $q = 0$. The result for the m'th order deformation equation is as follows:

$$L[u_m(T) - \chi_m u_{m-1}(T)] = hH(T)R_m[u_{m-1}(T)], \qquad (13)$$

where $\chi_m = 0$ for $m = 1$ and $\chi_m = 1$ for $m > 1$ and:

$$R_m[u_{m-1}(s)] = \frac{1}{(m-1)!}\left.\frac{\partial^{m-1}N[\phi(s;q),\alpha(q)]}{\partial q^{m-1}}\right|_{q=0}. \qquad (14)$$

Equation (13) with the initial conditions, $u_m(0) = \left.\dfrac{du_m}{dT}\right|_{T=0} = 0$, constitute a set of

hierarchical linear equations that should be successively solved to obtain u_m's. Moreover, in order to avoid the secular terms in the solution, all terms at the right-hand side of the Eq. (13) that contain $\cos(T)$ should be set to zero. This provides additional algebraic equations for ω_m's. The first order approximation of the frequency can then be obtained as follows:

$$\omega_0 = \frac{1}{2}\sqrt{\frac{\left(3\alpha_2 A^2 + 4p + 4\right)}{\left(A^2\alpha_1 + 1\right)}}. \tag{15}$$

The second and third order approximations of the frequency can also be determined by obtaining the solution for ω_1 and ω_2 as:

$$\omega_1 = \frac{-hA}{640\omega_0{}^3\left(A^2\alpha_1 + 1\right)}\Big[15\left(4\alpha_1\omega_0{}^2 - \alpha_2\right)\alpha_2 A^4 +$$

$$10\left(-4\alpha_1\omega_0{}^4 + 4\alpha_1\omega_0{}^2 + \alpha_2\omega_0{}^2 - 4p\alpha_2 - \alpha_2\right)A^2 \tag{16}$$

$$+\, 4p\omega_0{}^2 - 4p\Big],$$

$$\omega_2 = \frac{1}{12288\omega_0{}^5\left(A^2\alpha_1 + 1\right)}$$

$$[3h^2(64\alpha_1{}^3\omega_0{}^6 - 464\alpha_1{}^2\alpha_2\omega_0{}^4 +$$
$$148\alpha_1\alpha_2{}^2\omega_0{}^2 - 9\alpha_2{}^3)A^8 + 6h^2(160\alpha_1{}^2\omega_0{}^6 +$$
$$16p\alpha_1{}^2\omega_0{}^4 - 160\alpha_1{}^2\omega_0{}^4 - 272\alpha_1\alpha_2\omega_0{}^4 +$$
$$168\alpha_1 p\alpha_2\omega_0{}^2 + 80\alpha_1\alpha_2\omega_0{}^2 + 58\alpha_2{}^2\omega_0{}^2$$
$$-7p\alpha_2{}^2 - 10\alpha_2{}^2)A^6 - 288h\alpha_2\omega_0{}^2(4\alpha_1\omega_0{}^2 +$$
$$8\alpha_1\omega_0\omega_1 - \alpha_2)A^5 - 8h^2(-108\alpha_1\omega_0{}^6 +$$
$$112p\alpha_1\omega_0{}^4 + 32p^2\alpha_1\omega_0{}^2 + 120\alpha_1\omega_0{}^4 +$$
$$27\alpha_2\omega_0{}^4 - 112p\alpha_1\omega_0{}^2 - 172p\alpha_2\omega_0{}^2 - \tag{17}$$
$$32p^2\alpha_2 - 12\alpha_1\omega_0{}^2 - 30\alpha_2\omega_0{}^2 + 28p\alpha_2$$
$$+3\alpha_2)A^4 + 192h\omega_0{}^2(4\alpha_1\omega_0{}^4 + 16\alpha_1\omega_0{}^3\omega_1$$
$$-4\alpha_1\omega_0{}^2 - 8\alpha_1\omega_0\omega_1 - \alpha_2\omega_0{}^2 - 2\alpha_2\omega_0\omega_1 +$$
$$6p\alpha_2 + \alpha_2)A^3 - 32(27h^2 p\omega_0{}^4 + 192\alpha_1\omega_0{}^4\omega_1{}^2$$
$$+2h^2 p^2\omega_0{}^2 + h^2 p^3 - 30h^2 p\omega_0{}^2 - 2h^2 p^2$$
$$+3h^2 p)A^2 - 768hp\omega_0{}^2(\omega_0{}^2 + 2\omega_0\omega_1 - 1)A$$
$$-6144\omega_0{}^4\omega_1{}^2].$$

Equations (16 and 17) contain the auxiliary parameter, h, which is not yet assigned a value. This parameter does in fact, have a considerable influence on the convergence of the solution and should be determined such that its variation has a minimal effect on the variation of the solution. To do this, the so-called h-curves should be plotted for specific values of system parameters. Then the proper value for h can be chosen from the region where the slope of the $\omega - h$ curve is near to zero. This, however, may impose difficulty in plotting the frequency-amplitude curves, especially when h changes with A, since the proper value of h will not be the same for different values of A and thus the $\omega - h$ curves should be successively inspected to determine the proper value of h for each oscillation amplitude. To avoid this, the Pade approximant of the HAM solution can be used by following procedure of the homotopy-Pade technique. This technique applies the Pade approximation to the power series solution of the HAM obtained by Taylor's series expansion of the solution in q. The Pade approximation may be viewed as the generalization of Taylor's series, which uses the ratio of two polynomials to approximate a function (say f) as [25]:

$$f(q) \approx \frac{P_m(q)}{Q_n(q)} \tag{18}$$

where $P_m(q)$ and $Q_n(q)$ are polynomial functions of degrees m and n respectively. This approximant has usually faster convergence rates than Taylor's series. Moreover, in cases that f is Taylor's series, its Pade approximant may considerably accelerate the convergence. This is also true for the power series expansion of $\Omega(q)$ given in Eq. (10), whose [m,n] Pade approximant can be written as [16]:

$$\Omega(q) = \frac{\displaystyle\sum_{k=0}^{m}\tilde{A}_{m,k}(T)q^k}{\displaystyle\sum_{k=0}^{n}\tilde{B}_{n,k}(T)q^k}, \tag{19}$$

where $\tilde{A}_{m,k}$ and $\tilde{B}_{n,k}$ may be computed by different algorithms such as the qd-algorithm and the algorithm of Gragg [26]. In the present study, they are obtained by the Maple software. The embedding parameter q is then set to unity

in Eq. (19) leading to the final solution for ω based on the $[m,n]$ Pade approximant. For the present problem, the [1, 1] homotopy-Pade for the third order HAM solution, can be obtained as follows:

$$\omega = \omega_0 + \frac{\omega_1^2}{\omega_1 - \omega_2}, \tag{20}$$

the [2, 2] homotopy-Pade approximant corresponding to the fifth order HAM solution is also obtained as follows:

$$\omega = \omega_0 + \frac{\omega_1^2(\omega_3 - \omega_4) + \omega_1\omega_2(2\omega_3 - \omega_2) - \omega_2^3}{\omega_1(\omega_3 - \omega_4) + \omega_2(\omega_3 + \omega_4 - \omega_2) - \omega_3^2} \tag{21}$$

where ω_0, ω_1, ω_2, ω_3 and ω_4 are given in Eqs. (15-17).

4. Numerical results

Numerical results are presented in this section for the steady state frequency response of the beam, using different-order approximations of the HAM solution and are compared with the numerical solution. Numerical simulations are performed by the Maple software, which uses the Fehlberg fourth-fifth order Runge-Kutta method (rkf45). The properties of the beam considered here are, $EI = 76.29$ Ib s, $\rho = 0.2888 \times 10^{-4}$ Ib s^2 / in^2, $l = 35.625$ in and $p = 0.014$. Moreover, the numerical values of θ, α_1, α_2 and β corresponding to these properties are given in Table 1.

Table 1. Numerical values of the parameters in Eq.(3) for the beam with $EI = 76.29$ Ib s, $\rho = 0.2888 \times 10^{-4}$ Ib s^2 / in^2 and $l = 35.625$ in [1].

Mode Number	1	2	3	4
β	1.8751	4.6941	7.8548	10.99554
α_1	1.182	3.4556	8.2535	16.6
α_2	5.5	1.4623	1.189	1.123
θ^4	3.7813	438.3	3670.64	143361.1
p	0.014	0.014	0.014	0.014

Finding a proper value for the auxiliary parameter, h, is crucial for accurate prediction of the response by the HAM. For this purpose,

the variation of the non-dimensional frequency, ω, corresponding to the first two modes, are depicted in Figs. 2-3. Results are obtained for two different values of vibration amplitudes, A. The best values of h correspond to the regions where the rate of change of ω with h is zero. These values, however, are not the same for different amplitudes even when the seventh-order approximation is used. Similar results are also obtained for higher modes of vibration which confirms the necessity of using the homotopy-Pade technique for obtaining a unique expression for different vibration amplitudes.
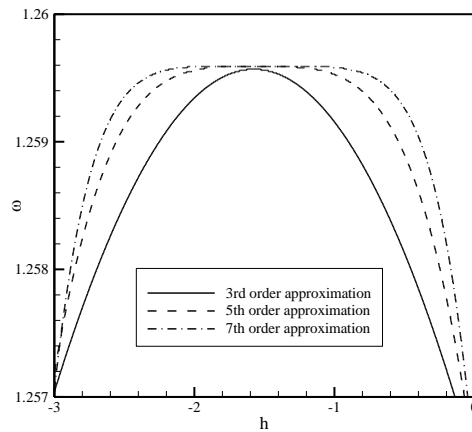
$A = 0.5$
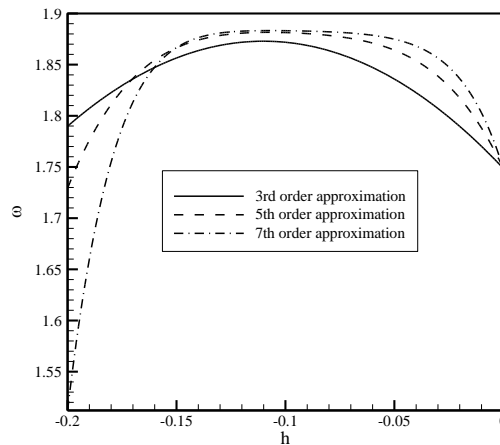
$A = 2$

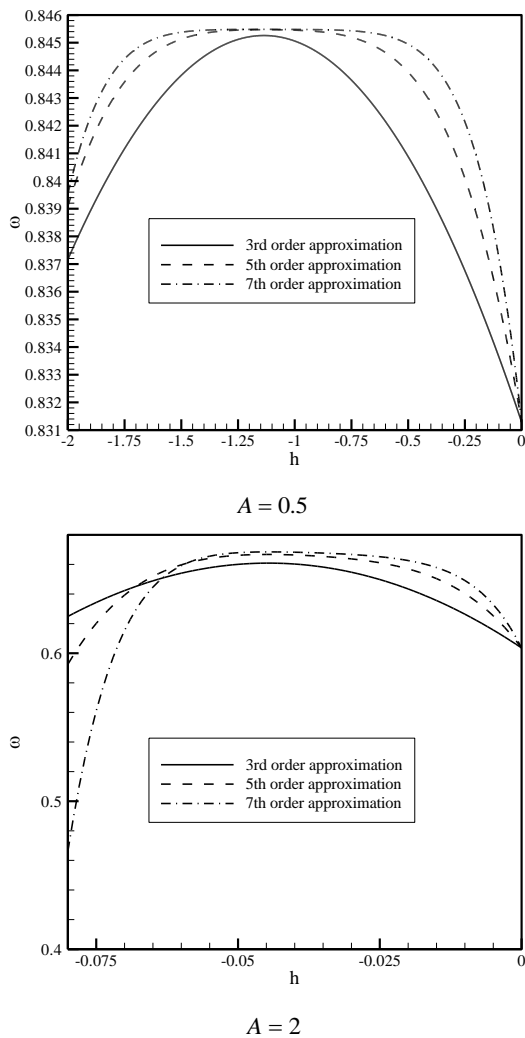**Fig. 2.** Variation of excitation frequency with auxiliary parameter h for the first mode.

become closer to the numerical solution, even though that h is chosen based on the ω-h curve obtained for $A = 2$. Considering the close agreement of the third order HAM with the numerical result at $A = 2$, it may be expected that better result may have been obtained if h is separately determined for different values of A. However, this would considerably slow down the process of obtaining the whole frequency-amplitude curve. Instead, the homotopy-Pade technique is used here which is found to have a minimal dependence on h. In fact, the solution is found to be varied with h in a narrow region near $h = 0$. So taking an arbitrarily large value for h, say $h = -10$, a unique expression can be obtained for all values of A. This is evident in Figs. 4-6, which shows excellent agreements between the [1, 1] homotopy-Pade and the numerical result, especially for the first and fourth modes. Slight discrepancy, however, exists for the second and third modes when $A > 1.5$, which has completely disappeared by using the [2, 2] homotopy-Pade technique. It must be mentioned here that the oscillation with $A > 1$ is strongly nonlinear and the high accuracy of the solution obtained by the homotopy-Pade technique for this range of vibration amplitudes, completely confirm the significant power of the analytical method.
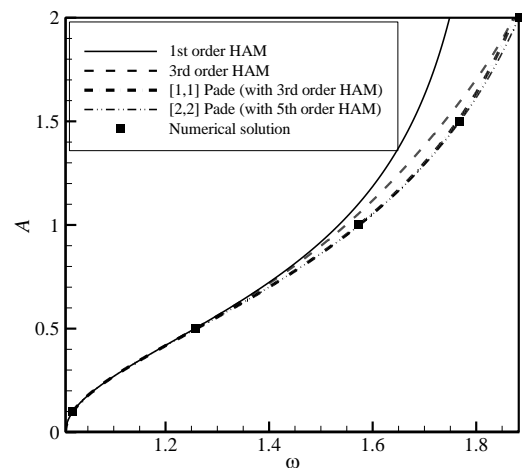

$A = 0.5$

$A = 2$

Fig. 3. Variation of excitation frequency with auxiliary parameter h for the second mode.

Next, the accuracy of the analytical solution is examined in Figs. 4-7 by depicting the frequency-amplitude curves for the first four modes of vibration. Analytical results, based on the HAM, are obtained using the proper values of h for $A = 2$. The results of the [1, 1] homotopy-Pade technique and also the [2, 2] Pade approximant of the fifth order HAM solution are also included in these figures. It can be seen that the first order approximation is adequate for the first mode's amplitudes smaller than 0.5. For higher modes, the nonlinearity seems to become stronger and thus the first order HAM is only accurate for $A < 0.3$. The third order approximation is also seen to

Fig. 4. Amplitude-frequency curve for the first mode ($h = -0.1$ for the third order HAM).

5. Conclusions

The HAM and the homotopy-Pade technique were used to obtain an accurate and efficient analytical solution for the nonlinear vibration of a parametrically excited cantilever beam. An explicit expression was presented for the third order approximation of the amplitude-frequency of the system. It was found that proper values of the auxiliary parameter, h, change with the non-dimensional vibration amplitude, A, making the HAM not suitable for the rapid depiction of the frequency-amplitude curves. The homotopy-Pade technique was thus employed, which besides improving the convergence rate, gave the solution that was almost independent of the auxiliary parameter h. The numerical results were presented for different modes of vibration, using both the HAM and homotopy-Pade technique and compared with the numerical solution. Highly accurate results were obtained using the [1, 1] Pade approximant of the third order HAM for non-dimensional amplitudes smaller than 1.5. For larger amplitudes up to 2, the [2, 2] Pade approximant of the fifth order HAM was found to coincide with the numerical solution, showing the significant power of the method in solving oscillatory equations with the strong nonlinearity.

Fig. 5. Frequency-Amplitude curve for the second mode ($h = -0.05$ for the third order HAM).

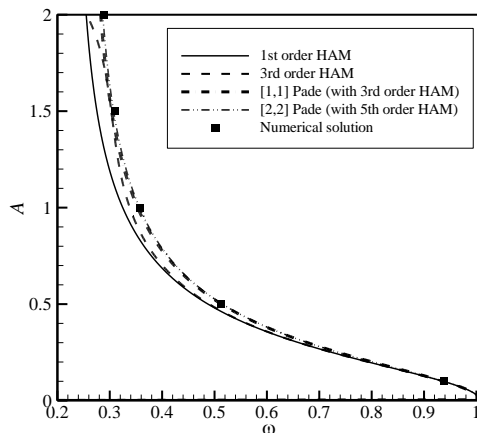**Fig. 6.** Frequency-Amplitude curve for the third mode ($h = -0.03$ for the third order HAM).

Fig. 7. Frequency-Amplitude curve for the fourth mode ($h = -0.015$ for the third order HAM).

References

[1] M. N. Hamdan, A. A. Al-Qaisia, "B. O. Al-Bedoor, Comparison of analytical techniques for nonlinear vibrations of a parametrically excited cantilever", *International Journal of Mechanical Sciences*, Vol. 43, No. 6, pp. 1521-1542, (2001).

[2] E. C. Haight, W.W. King, "Stability of nonlinear oscillations of an elastic rod", *Journal of the Acoustical Society of America*, Vol. 52, No. 3B, pp. 899-911, (1971).

[3] R. S. Haxton, A. D. S. Barr, "The autoparametric vibration absorber", *Transactions of the ASME, Journal of Engineering for Industry*, Vol. 94, No. 1, pp. 119-23, (1972).

[4] K. Sato, H. Saito, K. Otomi, "The parametric response of a horizontal beam carrying a concentrated mass under gravity", *Transactions of the ASME Journal of Applied Mechanics*, Vol. 44, No. 3, pp. 643-8, (1978).

[5] F.C. Moon, Experiments on chaotic motion of a forced nonlinear oscillator a strange attractors, *Journal of Applied Mechanics*, Vol. 47, No. 3, pp. 639-44, (1980).

[6] F. Pai, A.H. Nayfeh, "Nonlinear non-planar oscillations of a cantilever beam under lateral base excitations", *Journal of Sound and Vibration,* Vol. 25, No. 5, pp. 455-74, (1990).

[7] L. D. Zavodney, A. H. Nayfeh, "The nonlinear response of a slender beam carrying a lumped mass to a principal parametric excitation: theory and experiment", *International Journal of Nonlinear Mechanics*, Vol. 24, No. 2, pp. 105-25, (1989).

[8] T. D. Burton, M. Kolowith, "Nonlinear resonance and chaotic motion in a flexible parametrically excited beam", *Proceedings of the Second Conference on Nonlinear Vibrations, Stability and Dynamics of Structures and Mechanisms*, Blacksburg, VA, (1988).

[9] H. M. Sedighi, K. H. Shirazi and A. Noghrehabadi, "Application of recent powerful analytical approaches on the non-linear vibration of cantilever beams", *International Journal of Nonlinear Sciences and Numerical Simulation*, Vol. 13, No. 7, pp. 487-494, (2012).

[10] H. M. Sedighi, K. H. Shirazi, "A new approach to analytical solution of cantilever beam vibration with nonlinear boundary condition", *Journal of Computational and Nonlinear Dynamics*, Vol. 7, No. 3, pp. 1-4, (2012).

[11] H. M. Sedighi, K. H. Shirazi, A. Reza and J. Zare, "Accurate modeling of preload discontinuity in the analytical approach of the nonlinear free vibration of beams", Proceedings of the Institution of Mechanical Engineers, Part C: *Journal*

of Mechanical Engineering Science, Vol. 226, No. 10, pp. 2474-2484, (2012).

[12] H. M. Sedighi, K. H. Shirazi, M. A. Attarzadeh, A study on the quantic nonlinear beam vibrations using asymptotic approximate approaches, *Acta Astronautica*, Vol. 91, pp. 245-250, (2013).

[13] H. M. Sedighi, A. Reza, "High precise analysis of lateral vibration of quintic nonlinear beam", *Latin American Journal of Solids and Structures,* Vol. 10, No. 2, pp. 441- 452, (2013).

[14] H. M. Sedighi, F. Daneshmand, "Nonlinear transversely vibrating beams by the homotopy perturbation method with an auxiliary term", Journal of Applied and Computational Mechanics, Vol. 1, No. 1, pp. 1-9 , (2015).

[15] S. J. Liao, *Beyond Perturbation: Introduction to the Homotopy Analysis Method*, Chapman & Hall/CRC Press, Boca Raton, (2003).

[16] S. J. Liao, K. F. Cheung, "Homotopy analysis of nonlinear progressive waves in deep water", *Journal of Engineering Mathematics,* Vol. 45, No. 2, pp. 105-116, (2003).

[17] S. J. Liao, A. Campo, Analytic solutions of the temperature distribution in Blasius viscous flow problems, *Journal of Fluid Mechanics*. Vol. 453, No. 1, pp. 411-425, (2002).

[18] T. Pirbodaghi, M. T. Ahmadian, M. Fesanghary, "On the homotopy analysis method for non-linear vibration of beams", *Mechanics Research Communications*, Vol. 36, No. 2, pp. 143-148, (2009).

[19] R. Wu, J. Wang, J. Du, Y. Hu, H. Hu, "Solutions of nonlinear thickness-shear vibrations of an infinite isotropic plate with the homotopy analysis method", *Numerical Algorithms*, Vol. 59, No. 2, pp. 213-226, (2012).

[20] M. Poorjamshidian, J. Sheikhi, S. Mahjoub-Moghadas, M. Nakhaie, "Nonlinear vibration analysis of the beam carrying a moving mass using modified

homotopy", *Journal of solid mechanics*, Vol. 6, No. 4, pp. 389-396, (2014).

[21] H. M. Sedighi, K. H. Shirazi, J. Zare, "An analytic solution of transversal oscillation of quintic non-linear beam with homotopy analysis method", *International Journal of Non-Linear Mechanics*, Vol. 47, No. 7, pp. 777-784, (2012).

[22] S. H. Hoseini, T. Pirbodaghi, M. T. Ahmadian, G.H. Farrahi, "On the large amplitude free vibrations of tapered beams: an analytical approach", *Mechanic Research Communication*, Vol. 36, No. 8, pp. 892-897, (2009).

[23] M. R. M. Crespo da Silva, C. C. Glynn, "Nonlinear flexural-flexural-torsional dynamics of inextensible beams, I: equations of motion", *Journal of Structural Mechanics*, Vol. 6, No. 4, pp. 437-48, (1978).

[24] A. H. Nayfeh, P. F. Pai, *Linear and Nonlinear Structural Mechanics*, John Wiley & Sons, Weinheim, (2004).

[25] E. B. Saff, R. S. Varga, *Pade and Rational Approximation*, Academic Press, New York, (1977).

[26] J. Kallrath., *On Rational Function Techniques and Pade Approximants. An Overview*, Report, Ludwigshafen, Germany, (2002).